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Parliament and of the Council of 9  
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MEMBER OF EOTA



## European Technical Assessment ETA-19/0628 of 2019/10/11

### I General Part

**Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011: ETA-Danmark A/S**

**Trade name of the construction product:**

KNAPP T-JOINT connectors

**Product family to which the above construction product belongs:**

Three-dimensional nailing plate

**Manufacturer:**

Knapp GmbH  
Wassergasse 31  
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**Manufacturing plant:**

Knapp GmbH  
Wassergasse 31  
A-3324 Euratsfeld

**This European Technical Assessment contains:**

12 pages including 2 annexes which form an integral part of the document

**This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of:**

Guideline for European Technical Approval (ETAG) No. 015 Three Dimensional Nailing Plates, April 2013, used as European Assessment Document (EAD).

**This version replaces:**

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## II SPECIFIC PART OF THE EUROPEAN TECHNICAL ASSESSMENT

### 1 Technical description of product and intended use

#### Technical description of the product

KNAPP T-JOINT connectors are one-piece, face-fixed connectors to be used in timber-to-timber connections.

The KNAPP T-JOINT connectors are made from stainless steel grade GX5CrNi 19-10 (AISI304) with minimum yield strength  $R_e$  of 175 MPa. Dimensions, hole positions and typical installations are shown in Annexes A and B.

### 2 Specification of the intended use in accordance with the applicable EAD

KNAPP T-JOINT connectors are intended for use in making connections in load bearing timber structures, as a connection between two solid timber or wood based members, where requirements for mechanical resistance and stability and safety in use in the sense of the Basic Works Requirements 1 and 4 of Regulation (EU) 305/2011 shall be fulfilled.

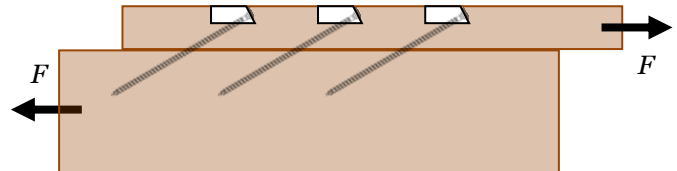
The KNAPP T-JOINT connectors can be installed as connections between wood based members such as:

- Structural solid timber of soft- or hardwood according to EN 338 / EN 14081,
- Glulam made of soft- or hardwood, classified according to EN 14080, or with ETA or national approval,
- Glued solid timber made of softwood, classified according to EN 14080, or with ETA or national approval,
- LVL according to EN 14374, or with ETA or national approval,
- Cross laminated timber and similar structural glued products according EN16351 or ETA,

However, the calculation methods are only allowed for a characteristic wood density of up to  $730 \text{ kg/m}^3$ . Even though the wood based material may have a larger density, this must not be used in the formulas for the load-carrying capacities of the fasteners.

Annex B states the formulas for the characteristic load-carrying capacities of the connections with KNAPP T-JOINT connectors. The design of the connections shall be in accordance with Eurocode 5 or a similar national Timber Code.

It is assumed that the forces acting on the connection are parallel to the interface between the timber members. The force  $F$  acts parallel to the axis of the connector and parallel to the timber member surface.



It is a condition for a force  $F$  that the T-Joint connector is connected to a wood-based member with screws in all holes marked.

The connectors are intended for use in connections subject to static or quasi static loading. The stainless steel connectors are for use in timber structures subject to conditions defined by the service classes 1, 2 and 3 of EN 1995-1-1:2008, (Eurocode 5). The screws to be used shall be made from suitable material.

Details of the corrosion protection system are deposited at ETA-Danmark.

The scope of the connectors regarding resistance to corrosion shall be defined according to national provisions that apply at the installation site considering environmental conditions.

The provisions made in this European Technical Assessment are based on an assumed intended working life of the connectors of 50 years.

The indications given on the working life cannot be interpreted as a guarantee given by the producer or Assessment Body, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

### 3 Performance of the product and references to the methods used for its assessment

Characteristic	Assessment of characteristic
<b>3.1 Mechanical resistance and stability*) (BWR1)</b>	
Characteristic load-carrying capacity	See Annex B
Stiffness	No performance assessed
Ductility in cyclic testing	No performance assessed
<b>3.2 Safety in case of fire (BWR2)</b>	
Reaction to fire	The connectors are made from steel classified as Euroclass A1 in accordance with EN 13501-1 and Commission Delegated Regulation 2016/364
<b>3.8 General aspects related to the performance of the product</b>	
Identification	See Annex A

\*) See additional information in section 3.8 – 3.9.

### 3.9 Methods of verification

The characteristic load-carrying capacities are based on the characteristic values of the connectors and the timber members.

According to EN 1990 (Eurocode – Basis of design) paragraph 6.3.5 the design value of load-carrying capacity can be determined by reducing the characteristic values of the load-carrying capacity with different partial factors.

Therefore, to obtain design values according to the Eurocodes or appropriate national codes of practice, the capacities have to be multiplied with different partial factors for the material properties and – for the connectors mounted in wood – also the coefficient  $k_{\text{mod}}$  that takes into account the load duration class.

Thus, the characteristic or design values of the load-carrying capacity are determined also for timber failure  $F_{\text{Rk,H}}$  (obtaining the embedment strength of connectors subjected to shear or the withdrawal capacity of the screw, respectively (see Annex B) as well as for steel failure of the screw  $F_{\text{tens,Rd}}$ . The design value of the load-carrying capacity is the smaller value of both load-carrying capacities.

$$F_{\text{Rd}} = \min \left\{ \frac{k_{\text{mod}} \cdot F_{\text{Rk,H}}}{\gamma_{\text{M,H}}}; F_{\text{tens,Rd}} \right\}$$

Therefore, for timber failure the load duration class and the service class are included. The different partial factors  $\gamma_{\text{M}}$  for steel or timber, respectively, are also correctly taken into account.

### 3.10 Mechanical resistance and stability

See annex B for characteristic load-carrying capacities of the connectors.

The characteristic capacities of the connectors are determined by calculation assisted by testing as described in the EOTA Guideline 015 clause 5.1.2. They should be used for designs in accordance with Eurocode 5 or a similar national Timber Code.

The design models allow the use of fasteners described in Annex A:

#### *Screws in accordance with EN 14592 or ETA*

In the formulas in Annex B the capacities for self-drilling screws calculated from the formulas of Eurocode 5 are used when calculating the axial screw load-carrying-capacity.

No performance has been assessed in relation to connection stiffness or ductility of a joint under cyclic testing. The contribution to the performance of structures in seismic zones, therefore, has not been assessed.

### 3.11 Related aspects of serviceability

#### 3.11.1 Corrosion protection in service class 1, 2 or 3.

In accordance with ETAG 015 the stainless steel connectors are produced from corrosion resistant steel castings. The steel employed is GX5CrNi 19-10 according to EN 10283:2010-06 with minimum yield strength  $R_e$  of 175 MPa.

#### Connector joints

A connector joint is deemed fit for its intended use provided:

- Connectors are fastened to wood-based members by screws.
- There shall be screws in all marked holes as prescribed in Annex A.
- The characteristic capacity of the connector joint is calculated according to the manufacturer's technical documentation, dated 2019-08-09.
- The connector joint is designed in accordance with Eurocode 5 or an appropriate national code.
- There is no gap between the timber member surfaces or between the member surface and the connector.
- The cross section of the timber members at the connector joint shall have sharp edges  $e$ , i.e. it shall be without wane.
- The dimensions of the timber members shall be so large that the minimum fastener end and edge distances are observed.
- Screws to be used shall have a diameter, which fits the holes of the connectors.

## **4 Attestation and verification of constancy of performance (AVCP)**

### **4.1 AVCP system**

According to the decision 97/638/EC of the European Commission<sup>1</sup>, as amended, the system(s) of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) is 2+.

## **5 Technical details necessary for the implementation of the AVCP system, as foreseen in the applicable EAD**

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at ETA-Danmark prior to CE marking

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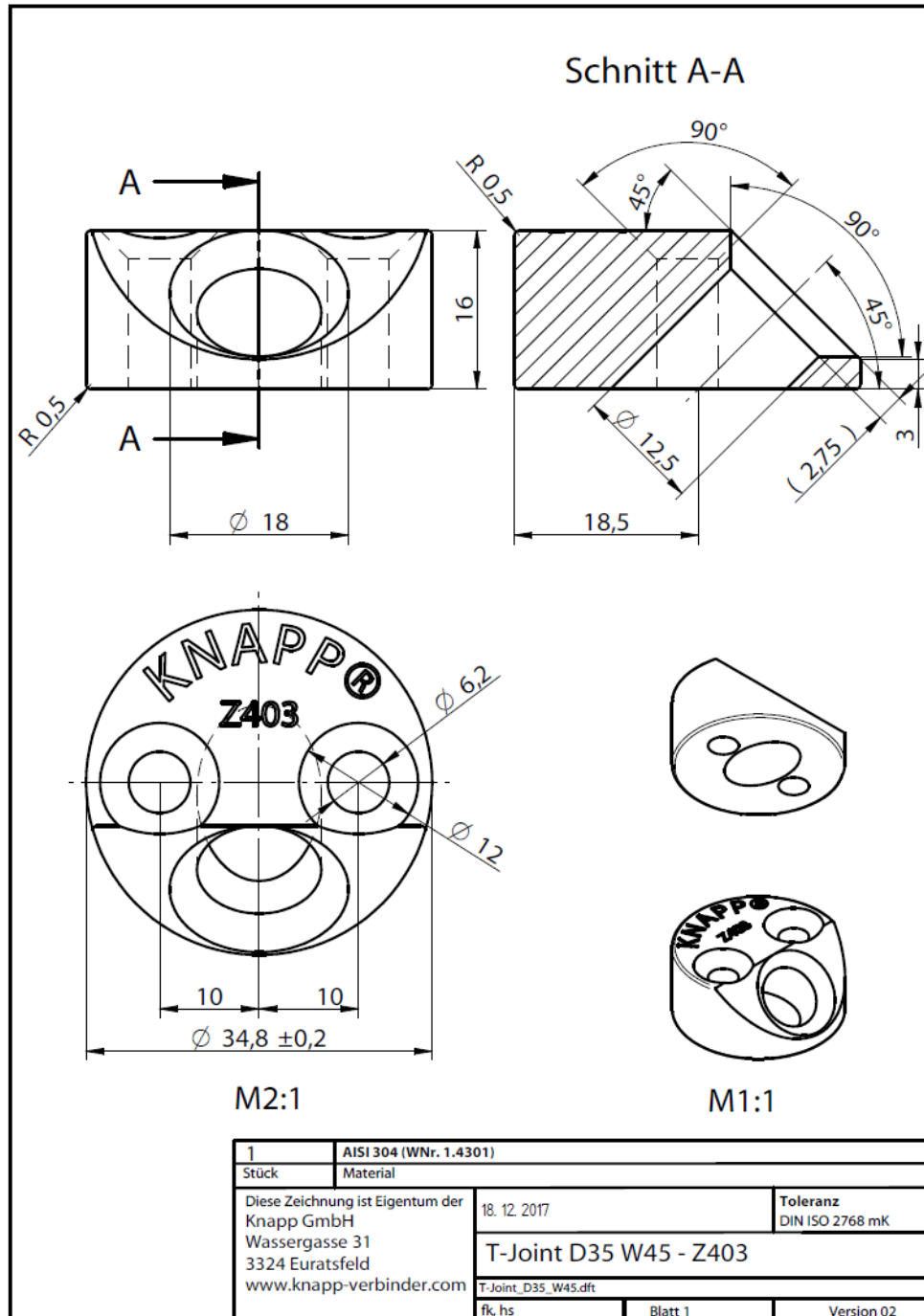


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### Annex A Product details and definitions

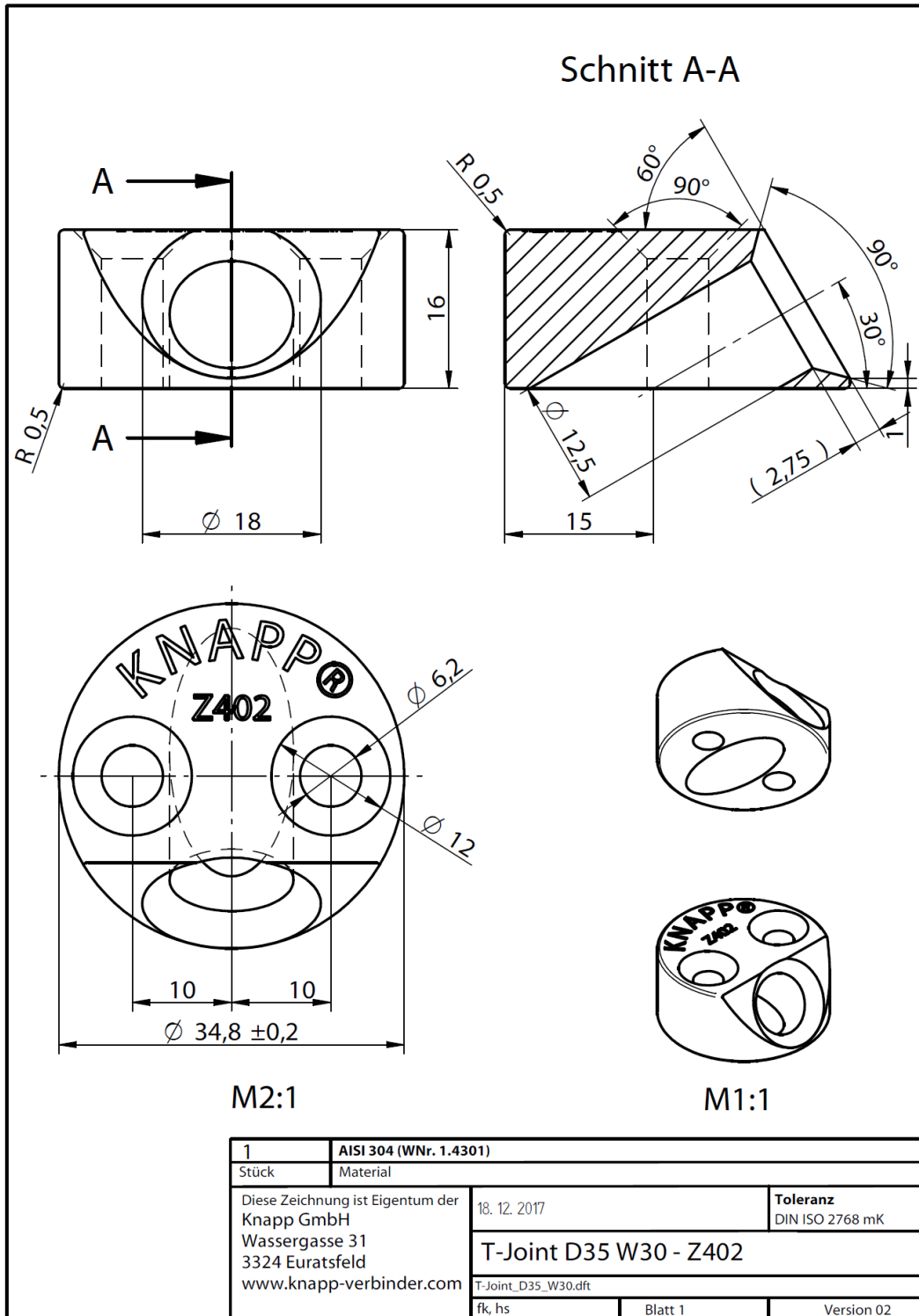
#### KNAPP T-JOINT connector D35/W45

Face mount one-piece connector. Steel casting for structural use GX5CrNi 19-10 according to EN 10283:2010-06 with minimum yield strength  $R_e$  of 175 MPa. Steel-to-timber connections with countersunk screw diameter 8 mm or 10 mm or 12 mm.



**KNAPP T-JOINT connector D35/W30**

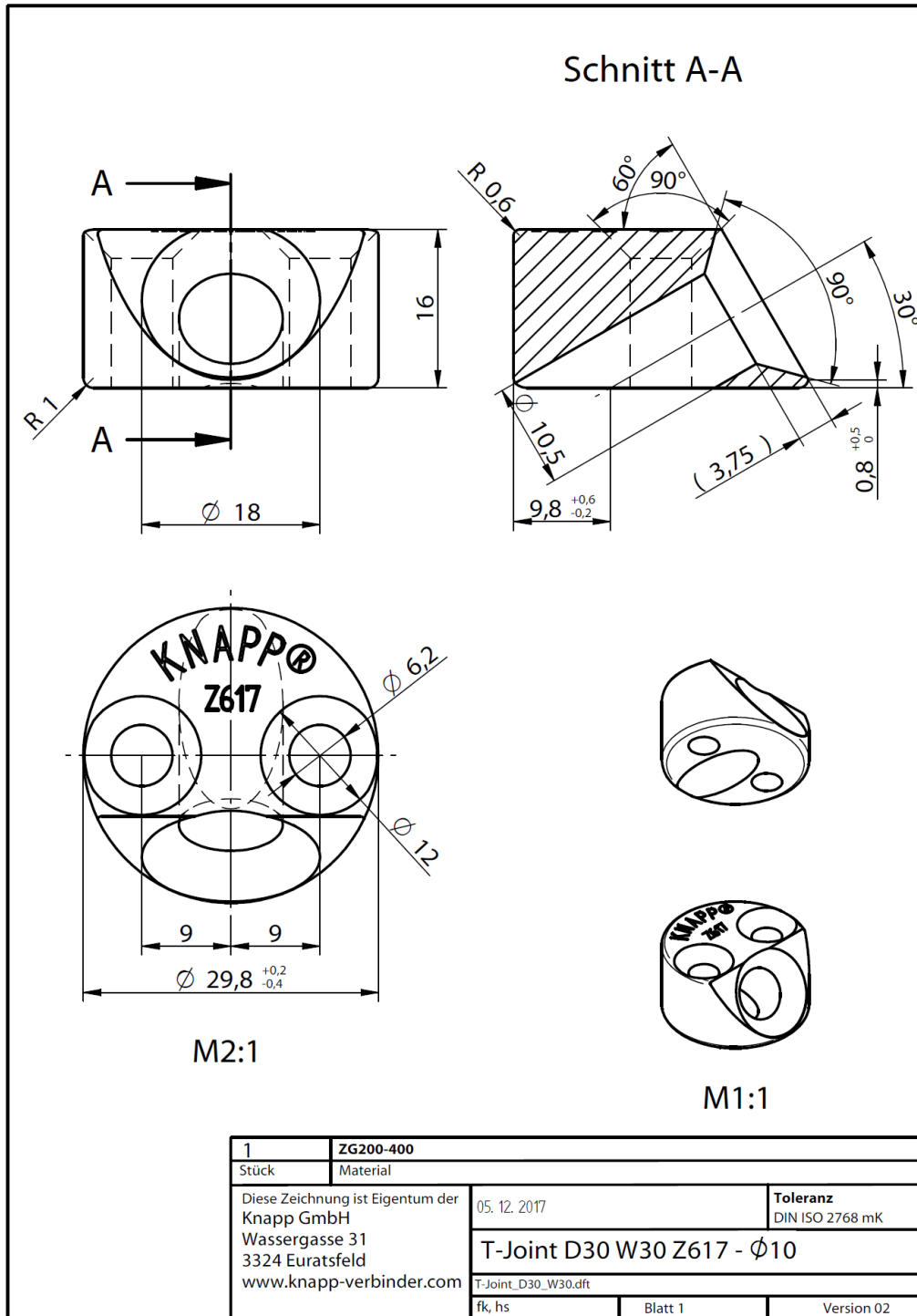
Face mount one-piece connector. Steel casting for structural use GX5CrNi 19-10 according to EN 10283:2010-06 with minimum yield strength  $R_e$  of 175 MPa. Steel-to-timber connections with countersunk screw diameter 8 mm or 10 mm or 12 mm.





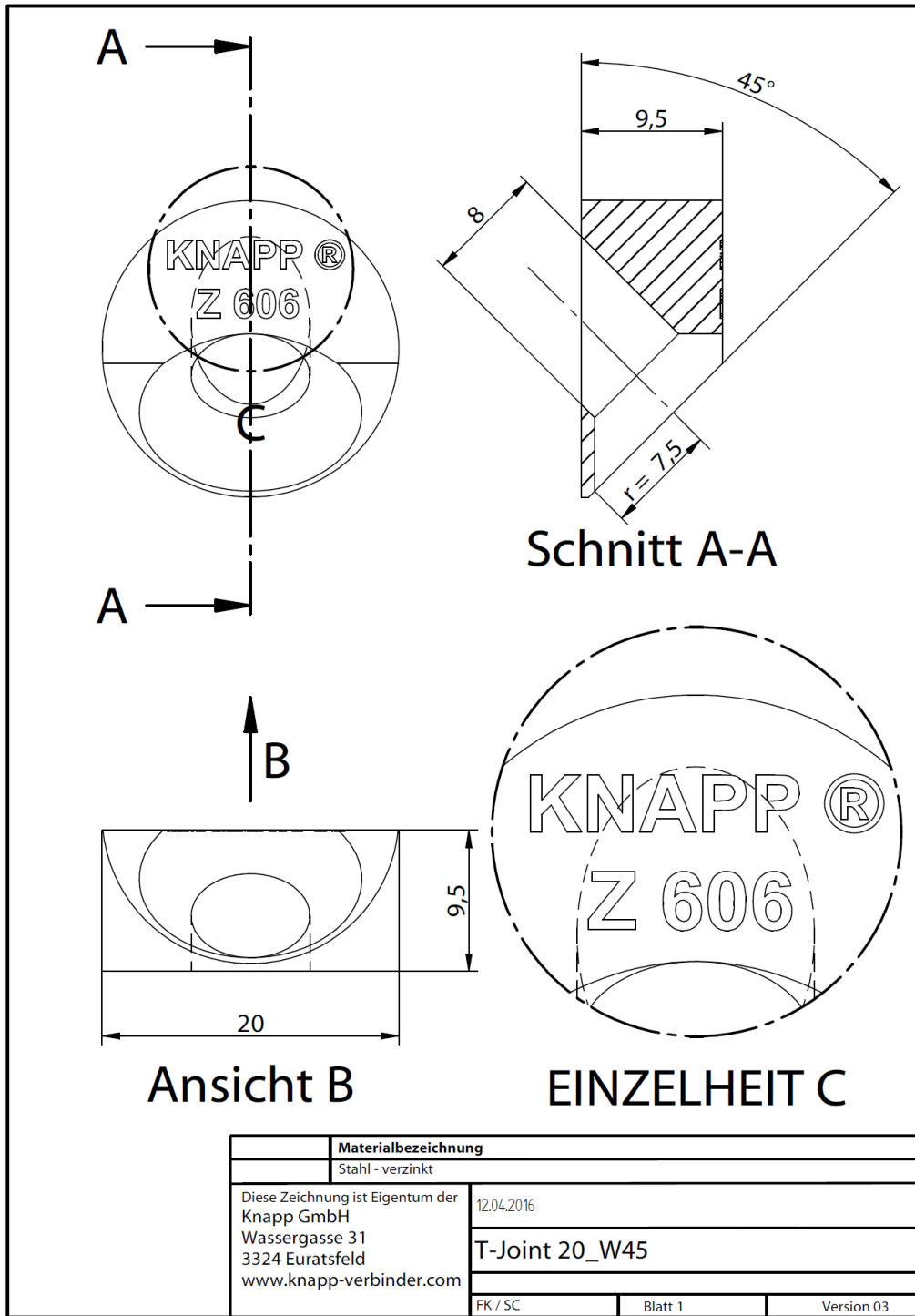
**KNAPP T-JOINT connector D30/W30**

Face mount one-piece connector. Steel casting for structural use GX5CrNi 19-10 according to EN 10283:2010-06 with minimum yield strength  $R_e$  of 175 MPa. Steel-to-timber connections with countersunk screw diameter 8 mm or 10 mm.



**KNAPP T-JOINT connector D20/W45**

Face mount one-piece connector. Steel casting for structural use GX5CrNi 19-10 according to EN 10283:2010-06 with minimum yield strength  $R_e$  of 175 MPa. Steel-to-timber connections with countersunk screw diameter 6 mm or 8 mm.

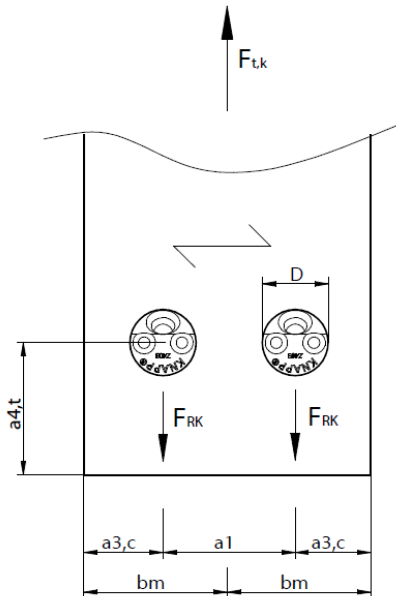


Fastener types and sizes

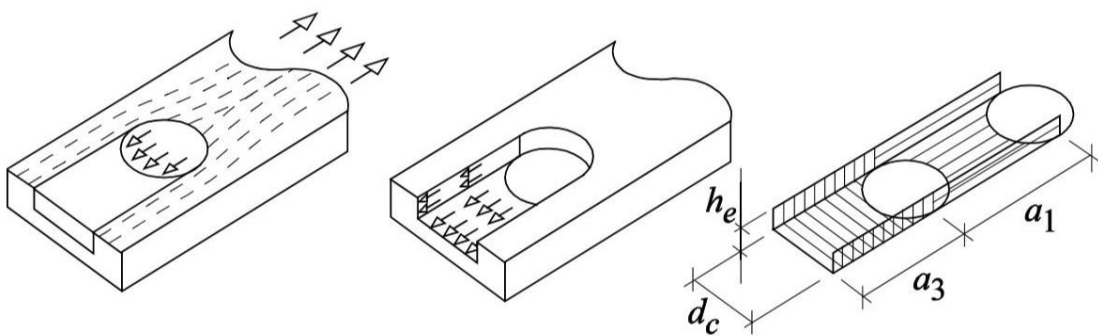
SCREW diameter [mm]	Length [mm]	Screw type
6.0 - 12.0	120 - 400	Self-tapping load-bearing screws according to EN 14592 or ETA
5.0 – 6.0	60 - 100	Fixing screws according to EN 14592 or ETA

Minimum spacings and edge and end distances for T-Joint connectors -see figures for legend.

Spacing and edge/end distances (see EN 1995-1-1 Figure 8.7)	Angle to grain	Minimum spacings and edge/end distances
$a_1$ (parallel to grain)	$0^\circ \leq \alpha \leq 360^\circ$	$2,0 D$
$a_2$ (perpendicular to grain)	$0^\circ \leq \alpha \leq 360^\circ$	$2,0 D$
$a_{3,t}$ (loaded end)	$-90^\circ \leq \alpha \leq 90^\circ$	$2,0 D$
$a_{3,c}$ (unloaded end)	$90^\circ \leq \alpha < 270^\circ$	$1,2 D$
$a_{4,t}$ (loaded edge)	$0^\circ \leq \alpha \leq 180^\circ$	$2,0 D$
$a_{4,c}$ (unloaded edge)	$180^\circ \leq \alpha \leq 360^\circ$	$1,2 D$



Legend for edge end distances

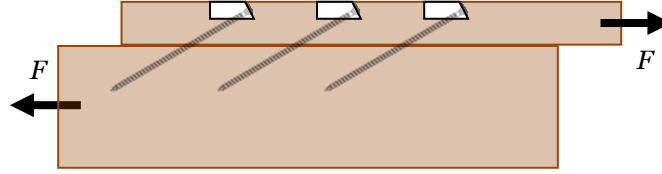


Shear areas in split ring connection

## Annex B

### Characteristic values of load-carrying-capacities

The forces are assumed to act parallel to the timber member surface. Only a full fastener pattern is specified, where there are screws in all the three holes of the connector except T-Joint D20/W45, where there is a single screw in the hole of the connector.



#### Loading parallel to grain:

$$F_{Rk} = \min \begin{cases} F_{ax,Rk} \cdot \cos \alpha \\ \frac{f_{head,k} \cdot D^2}{\tan \alpha} \left( \frac{\rho_k}{350} \right)^{0,8} \\ 0,09 \cdot \rho_k \cdot D \cdot h_e \\ K \cdot A_s^{0,75} \end{cases} \quad (B.1)$$

#### Loading perpendicular to grain:

$$F_{Rk} = \min \begin{cases} F_{ax,Rk} \cdot \cos \alpha \\ \frac{f_{head,k} \cdot D^2}{\tan \alpha} \left( \frac{\rho_k}{350} \right)^{0,8} \\ 0,07 \cdot \rho_k \cdot D \cdot h_e \\ f_{vr,k} \cdot a_{4,t} \cdot b_m \end{cases} \quad (B.2)$$

Where:

- $\alpha$  Angle between screw axis and member surface,  $\alpha = 30^\circ$  or  $\alpha = 45^\circ$ ;
- $F_{ax,Rk}$  Characteristic tensile or withdrawal capacity of load-bearing screw in N, the lower value governs;
- $f_{head,k}$  Characteristic pull-through parameter in  $N/mm^2$ ,  $f_{head,k} = 12 N/mm^2$ ;
- $D$  Outer diameter of T-Joint connector,  $D = 20$  mm or  $D = 30$  mm or  $D = 35$  mm;
- $\rho_k$  Characteristic density of timber member;
- $h_e$  Thickness of T-Joint connector,  $h_e = 9,5$  mm for  $D = 20$  mm or  $h_e = 16$  mm for  $D = 30$  mm or  $D = 35$  mm;
- $a_1$  Spacing between T-Joints parallel to grain direction in mm;
- $a_{3,t}$  Loaded end distance in mm;
- $a_{4,t}$  Loaded edge distance in mm;
- $K$  Factor;
  - $K = 20 N/mm^{1,5}$  for softwood solid timber, glued solid timber, glulam or CLT,
  - $K = 30 N/mm^{1,5}$  for softwood LVL,
  - $K = 40 N/mm^{1,5}$  for hardwood solid timber, glulam or CLT,
  - $K = 50 N/mm^{1,5}$  for hardwood LVL;
- $A_s$  Shear area;

$$A_s = a_{3,t} \cdot (2 \cdot h_e + D) - \frac{\pi \cdot D^2}{8} \text{ for connections with one T-Joint in load direction}$$

$$A_s = a_1 \cdot (2 \cdot h_e + D) - \frac{\pi \cdot D^2}{2} \text{ for connections with more than one T-Joint in load direction}$$

$f_{vr,k}$  Characteristic rolling shear strength;

$b_m$  Width of rolling shear area;  $b_m = 0,5 \cdot a_1 + a_{3,c}$  or  $b_m = a_1$ .